

Influence of angularity of coarse fraction grains on internal erosion process

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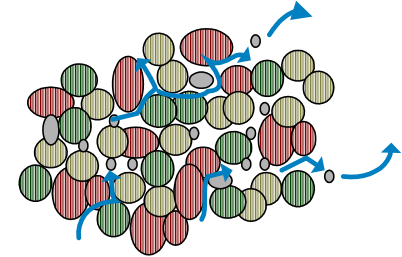


Background

Two types of internal erosion process in uncracked soils:

backward erosion and **suffusion**

Diffuse redistribution of **fine particles** within the soil.



Slight changes of grain size distribution and the **hydraulic conductivity** can evolve notably (Lafleur, 1999; Bendahmane et al. 2008; Marot et al. 2009)

Result from **seepage flow** in the soil pores →

great influence of **geometry of the interstitial vacuums**

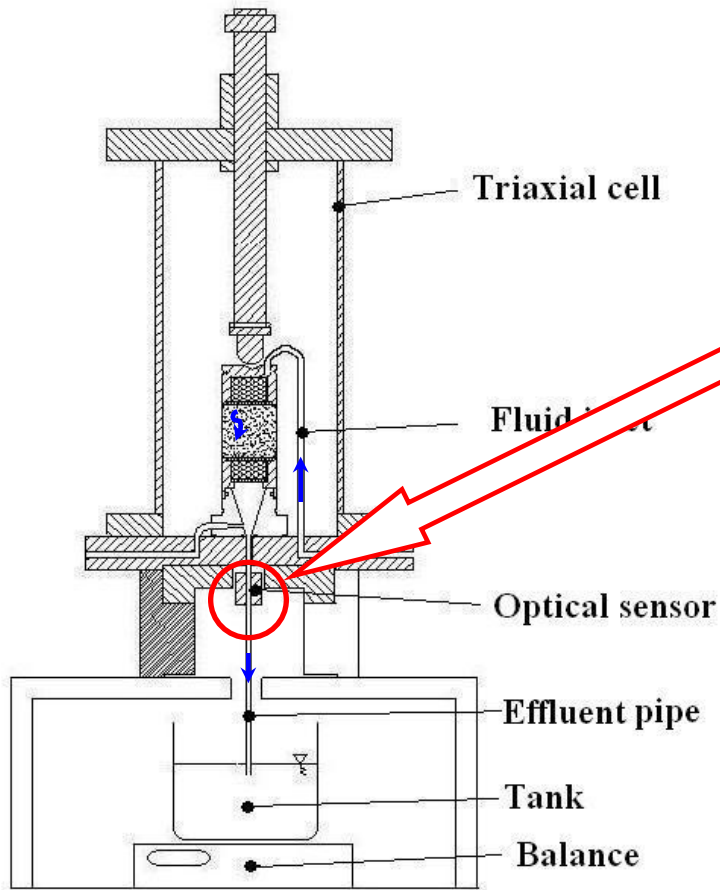
Granular assembly which depends on :

- **grain size distribution** (Wan and Fell, 2008; Li and Fannin, 2008, etc...)
- **mechanical loading conditions** (Skempton and Brogan, 1994; Moffat and Fannin 2011)
- **grain shape**

Average pore diameter as a function of shape coefficient (Kovacs, 1981)

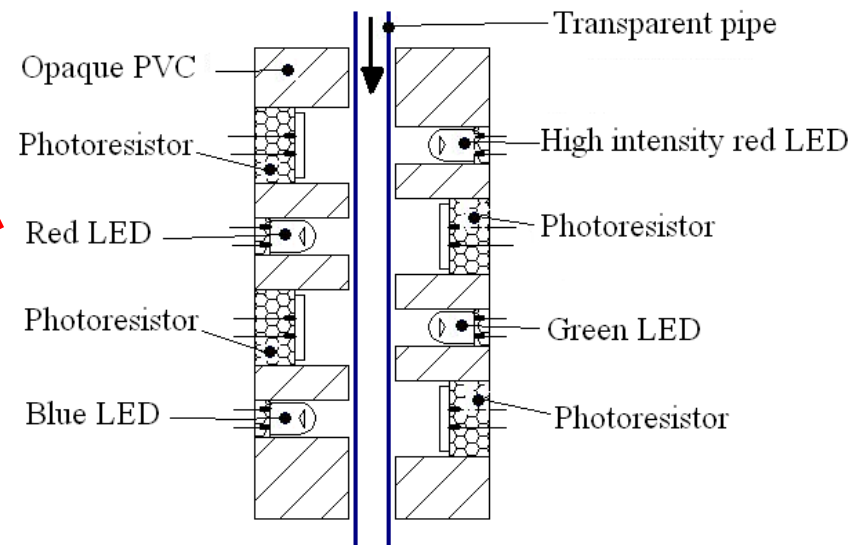
Experimental devices

Triaxial erodimeter for suffusion tests



Small quantity of eroded particles

Multichannel optical sensor



- measurement of fine particle concentration
- distinction between fines and sand grains



Optical and mechanical methods used for characterization of **grain angularity**

- **Optical microscope** (max enlargement: 40) associated with digital camera
→ Picture analysis

For each tested aggregate

$$\text{Roundness} = -\frac{P^2}{4.\pi.A}$$

$$\text{Circularity} = \frac{1}{n} \sum_{i=1}^n (r_i - r_{moy})^2$$

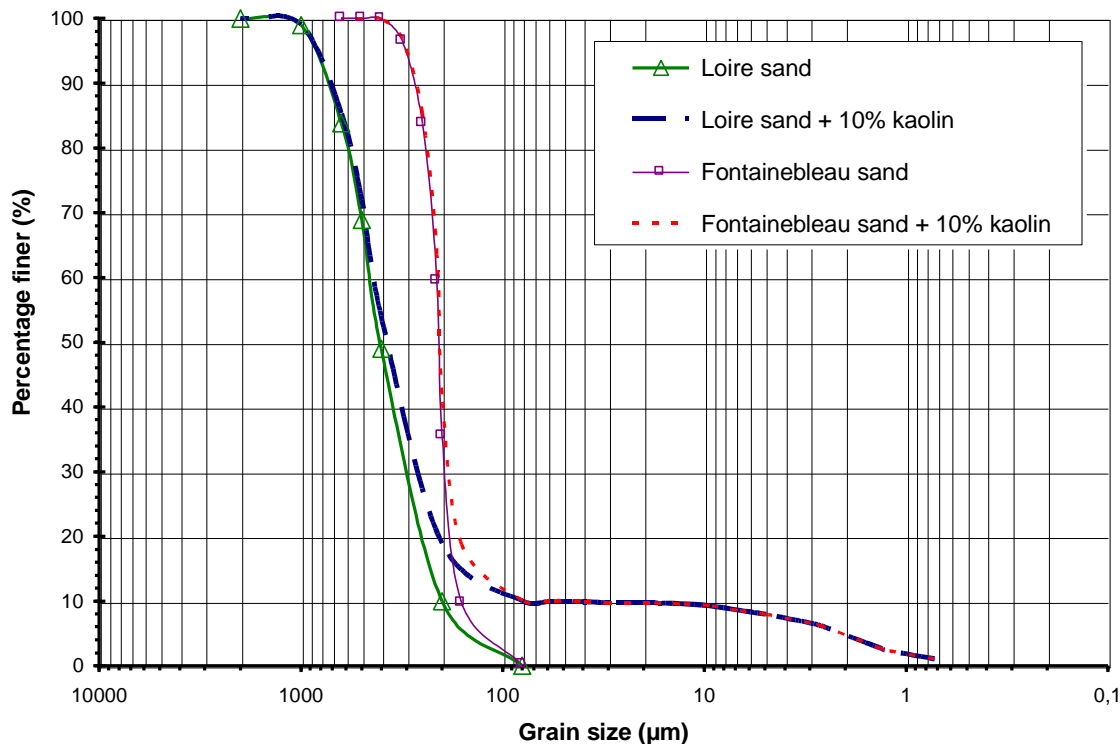
P: perimeter; A: cross section; r_i : radius; r_{moy} : average radius

- **Direct shear box** with density index I_d near to 1 ($I_d = [e_{max} - e]/[e_{max} - e_{min}]$)
→ internal friction angle
- **Angulometer**: standardized funnel, measurement of the gravitating flow duration (E_{cm}). Angularity index I_A : ratio of flow duration of aggregate to flow duration of glass beads

Tested materials

Mixtures of 10% of kaolin and 90% of one of the following aggregates:

- Fontainebleau sand (F) (75 μ m-425 μ m, D_{50} = 207 μ m, uniformity coefficient = 1.33)
- Loire sand (L) (80 μ m-1mm, D_{50} = 440 μ m, uniformity coefficient = 3.13)
- Modified Loire sand (LM) composed of grains from L, grain size distribution of F
- Glass beads grain size distribution of F



Laser diffraction
particle-size analyzer



Specimen preparation for suffusion tests

Preliminary forming of specimen by single layer semi-static compaction technique

Specimen placed on a 4mm pore opening grid → migration of all particles

Saturation phase with carbon dioxide , demineralized and deaerated water

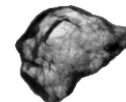
Consolidation (100kPa) → dry density 17.5 kN.m^{-3}

Suffusion test under constant hydraulic gradient

Angularity test results

Circularity very sensitive

Loire sand



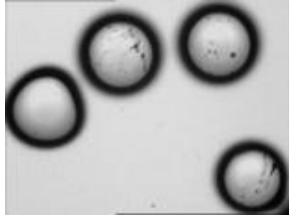
Circularity (μm^2)	25.87	263.03	2 217.32
Roundness (.)	1.19	1.4	1.9
Diameter (μm)	311	478	580

X 85
X 1.6

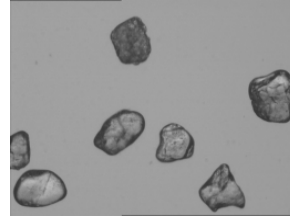


100 pictures for each aggregate

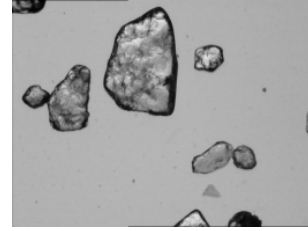
Glass bead



Fontainebleau (F)



Loire (L)



Circularity (μm^2)	3 (22)	63 (52)	178 (305)
Roundness (.)	1.13 (0.06)	1.39 (0.15)	1.42 (0.19)

Mean value (value of standard deviation)

Internal friction angle ($^\circ$)	30	37	44
Flow duration E_{cm} (s)	17	22	23
Index I_A (-)	1	1.29	1.35

→ Same relative classification

Glass beads: smallest values

Loire sand: largest values

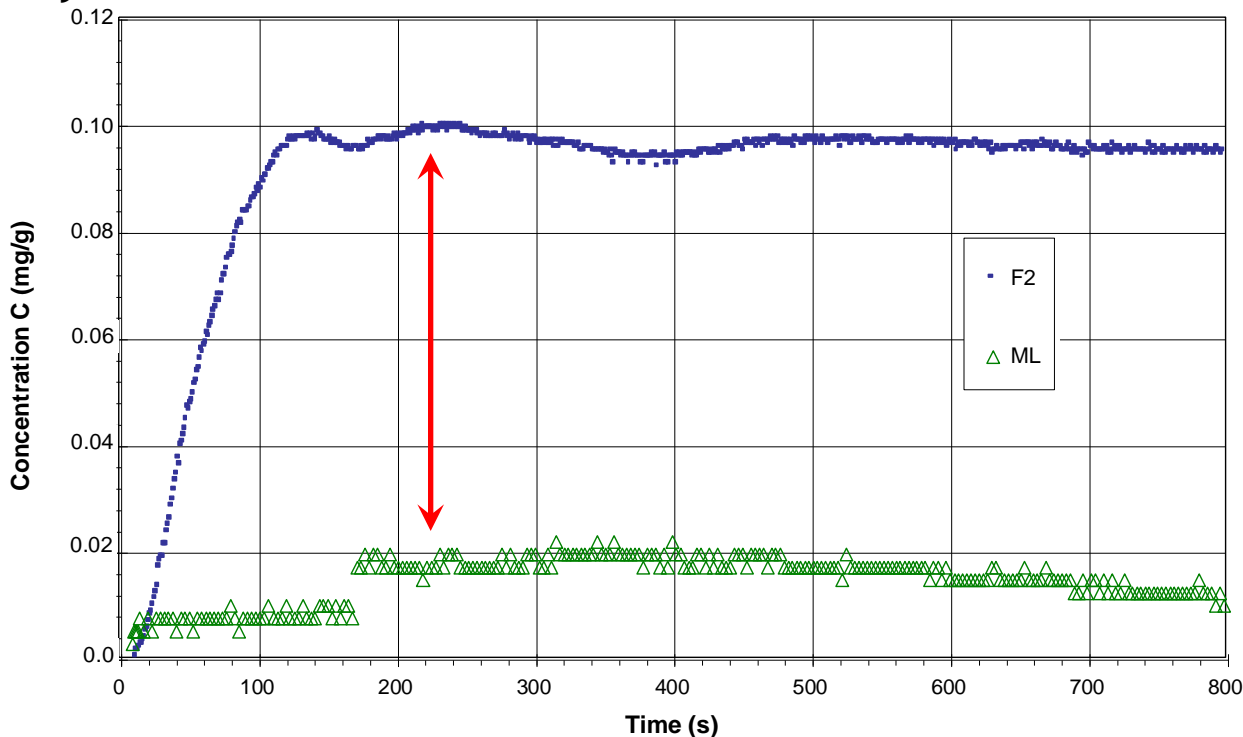
Fontainebleau sand: intermediate

Suffusion test results

Specimen constituted by kaolin and glass beads
→ instability during saturation phase

F and ML, **same grain size distribution**, same applied hydraulic gradient **$i=0.8$**

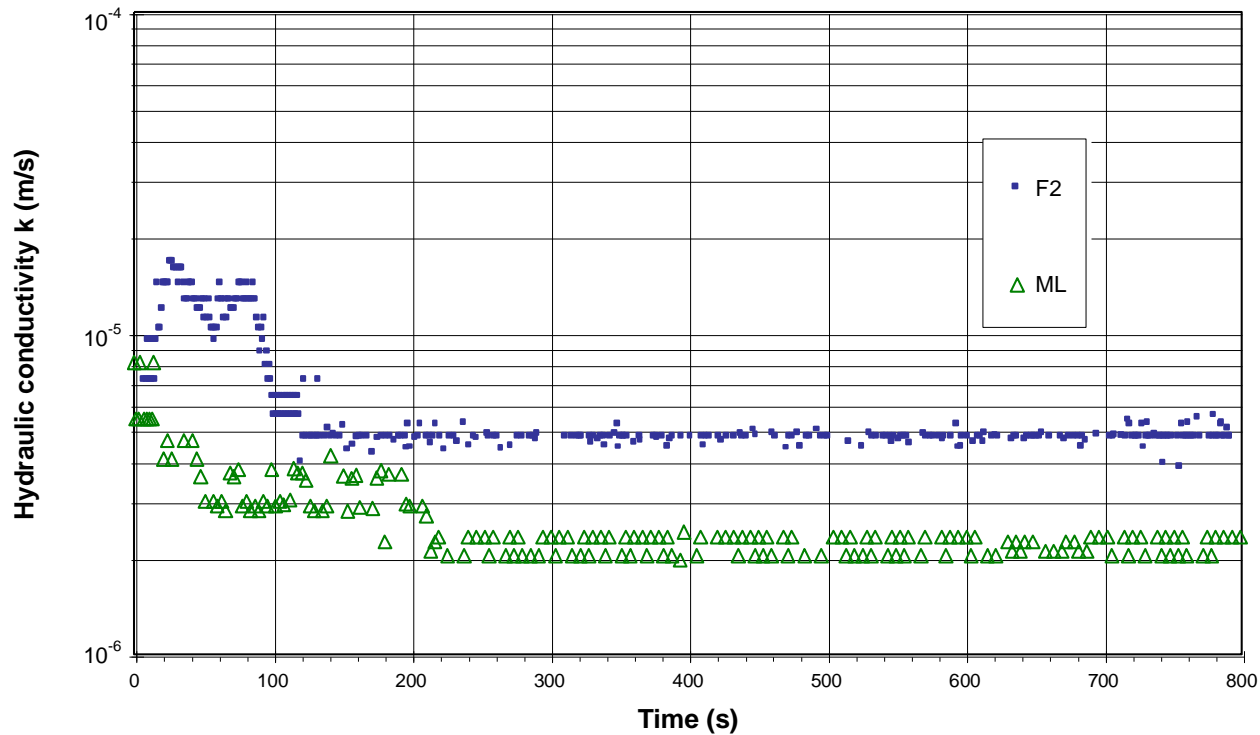
Clay concentration in effluent



→ Effect of grain shape: increase of resistance by a factor of 5



Hydraulic conductivity



ML: more important decrease of hydraulic conductivity

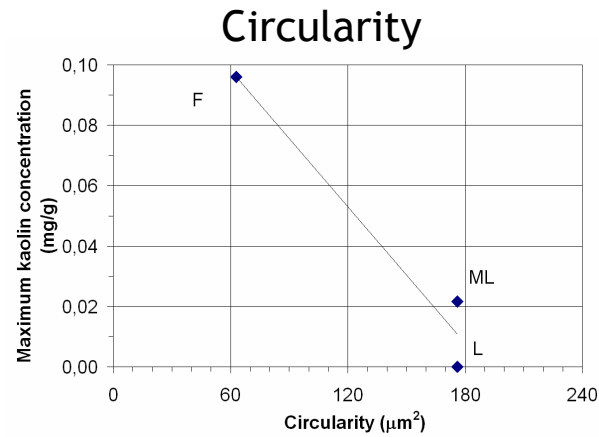
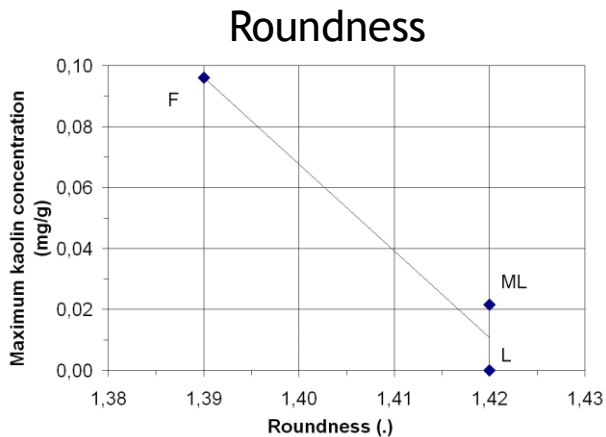
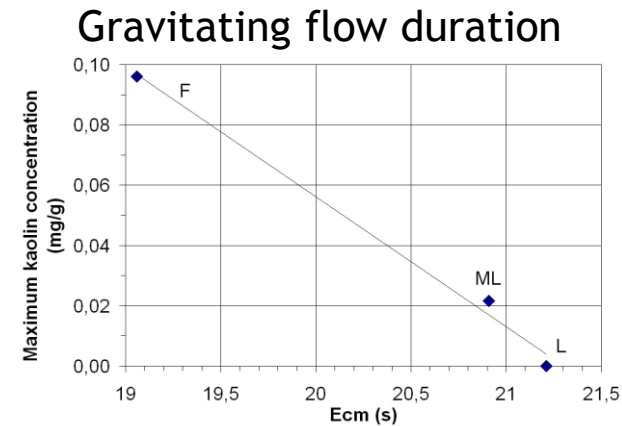
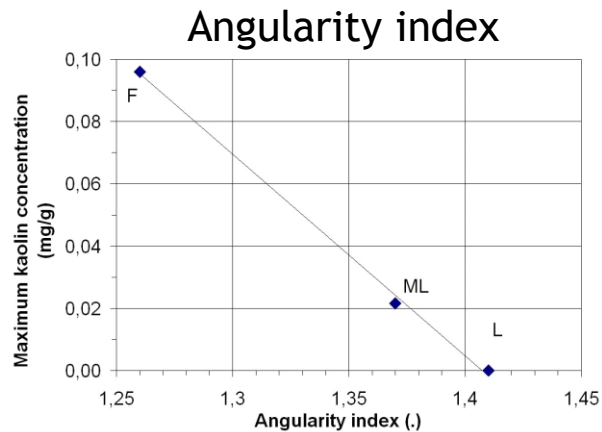
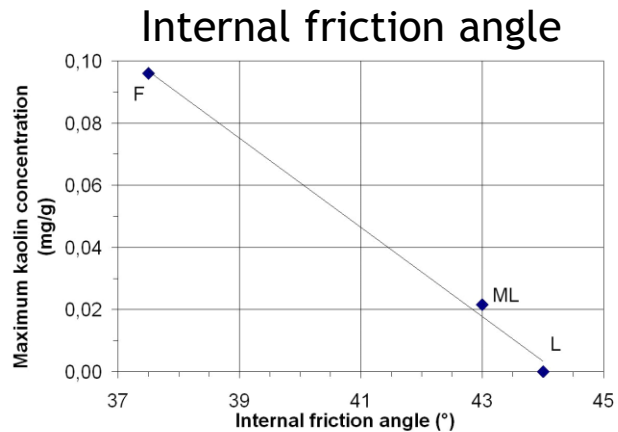
Decrease in hydraulic conductivity attributed to particle filtration

(Reddi et al., 2000; Bendahmane et al., 2008; Marot et al., 2009, 2011)

→ angularity seems to contribute to increase filtration process



Maximum clay concentration versus



Optical parameters no distinction between ML and L,
 great values of standard deviation → **Mechanical approach : preferred**



Thank you for your attention!

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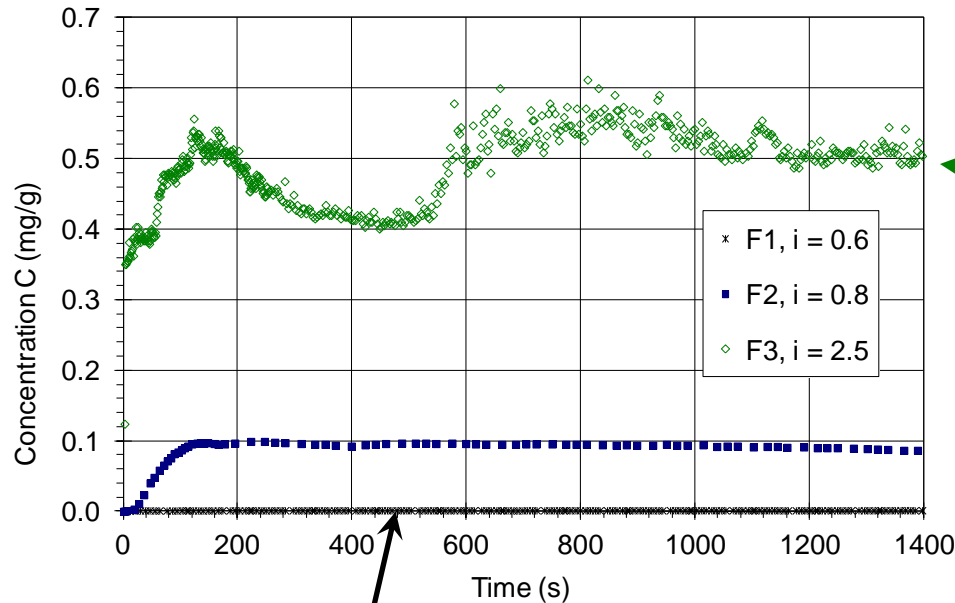


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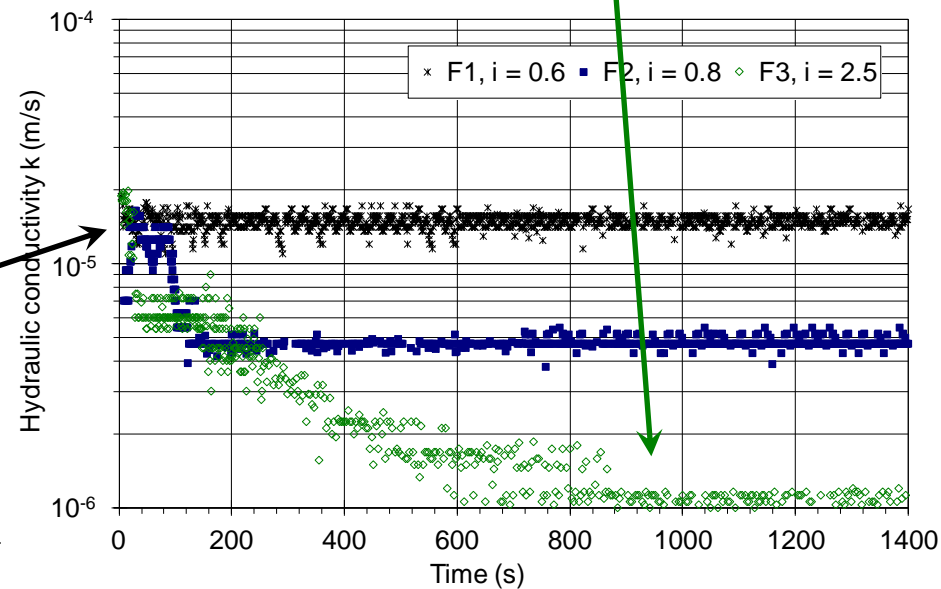
Suffusion test results

Mixture of 10% kaolin and 90% Fontainebleau sand



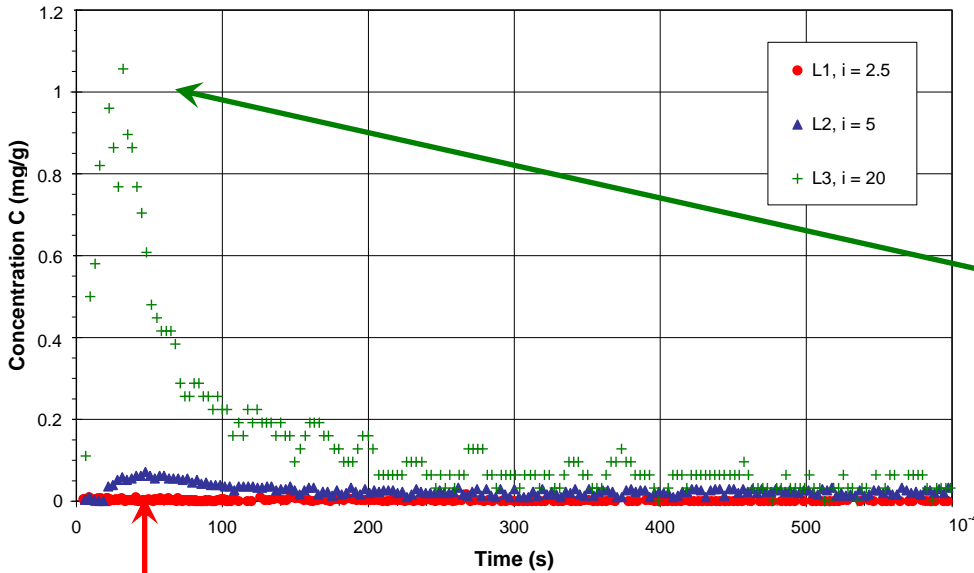
$i=0.6$
no erosion
hydraulic conductivity stays constant

$i=2.5$
 $C_{max}=0.55\text{mg/g}$
hydraulic conductivity decreases
from 10^{-5}m/s to 10^{-6}m/s



Suffusion test results

Mixture of 10% kaolin and 90% Loire sand



$i=20$
 $C_{\max} = 1.1 \text{ mg/g}$
 Important decrease in only a few minutes

$i=2.5$
 $C_{\max} < 10^{-2} \text{ mg/g}$
 hydraulic conductivity decreases from 10^{-5} m/s to 10^{-6} m/s

