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and Erosion”*

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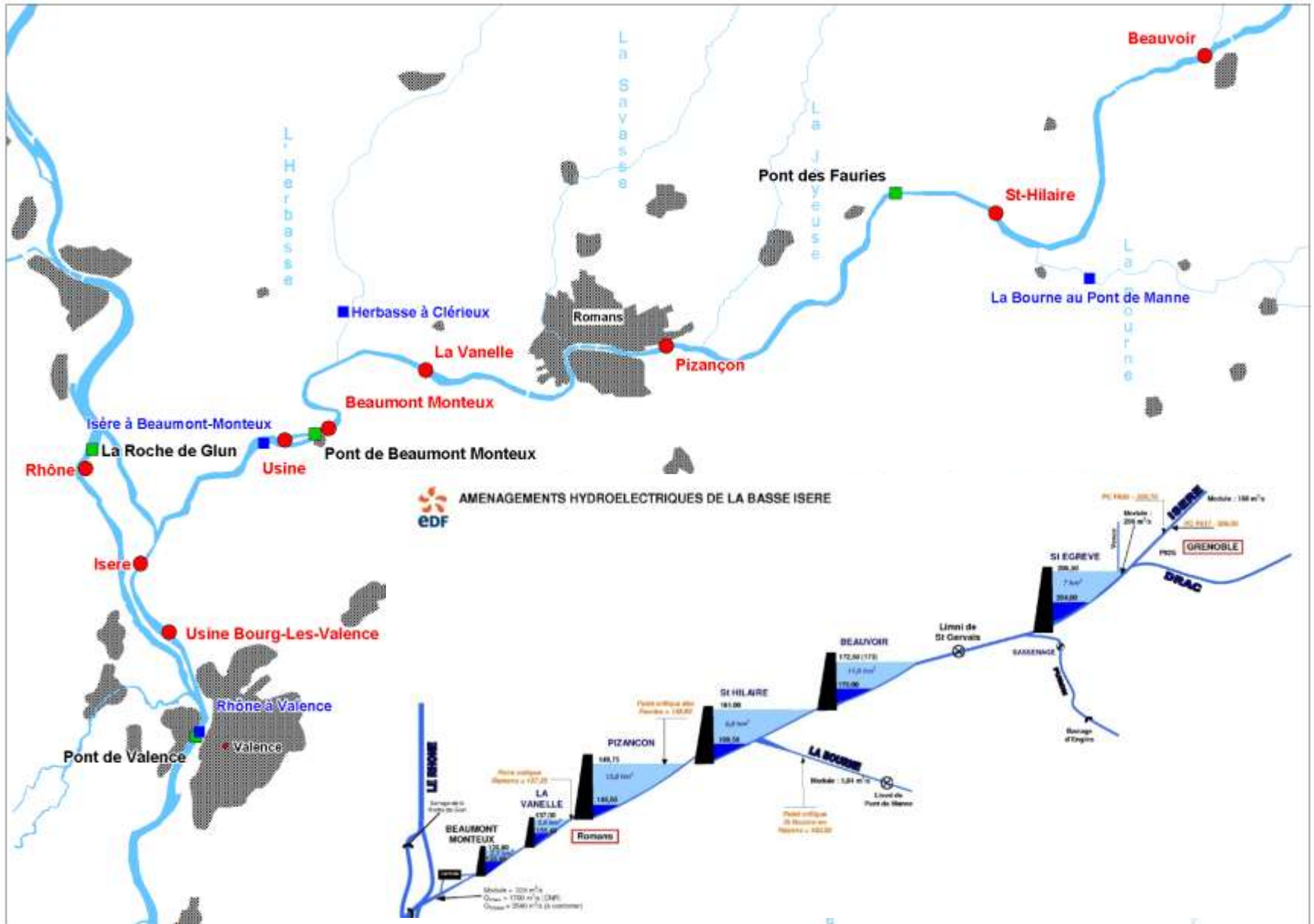


# **Improving sediment management in the reservoirs of the Lower Isère: a modelling-based approach**

# INTRODUCTION

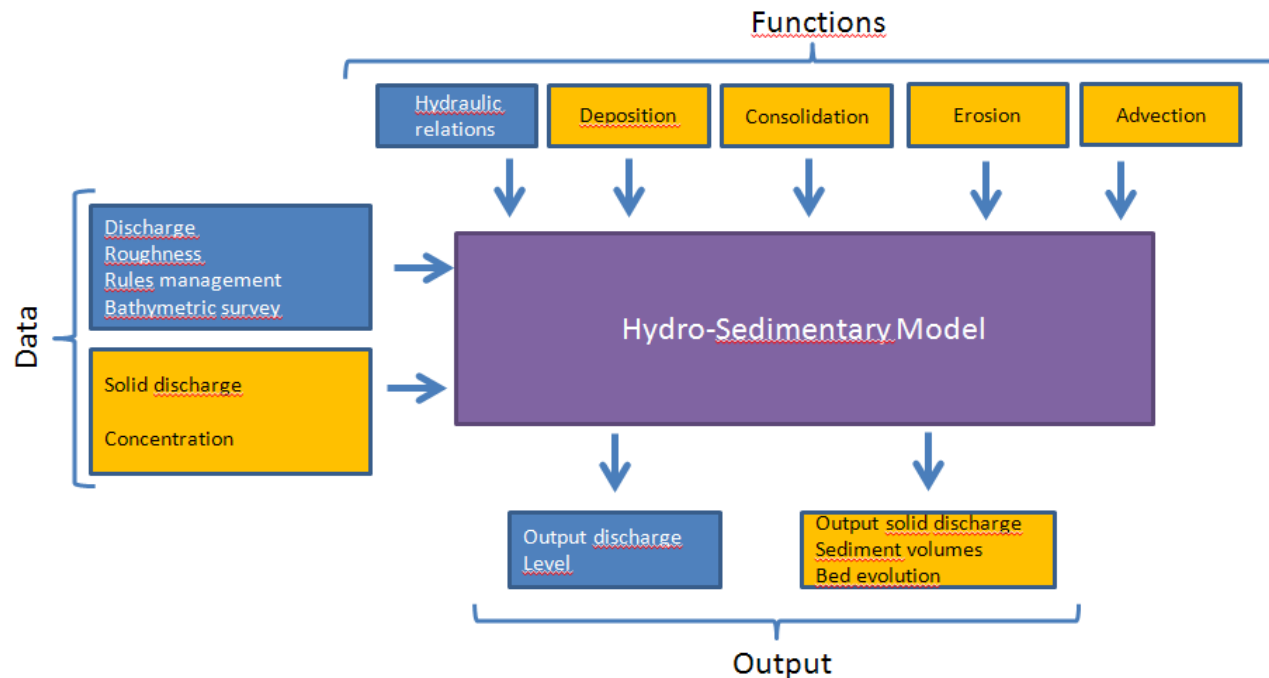


INTRODUCTION



## ■ Simplified and large-scale Hydro-Sedimentary Model

- ✓ Conceptual model covering 80 km
- ✓ Compute volume and sediment flows coherent with physical phenomena involved
- ✓ Assess management of large quantities of sediment (washload) passing through the reservoirs and in relation to the level and flushing management rules



▪ 2 models :

✓ Mascaret model for hydraulic

✓ Specific (Sedimasc) model for sedimentary calculations (in fact simplified deterministic modeling):

- shear stress due to friction forces :  $\tau = \rho_{\text{water}} g Rh J$
- Head slope :  $J = Q^2 / D^2$  , D being the discharge capacity

if  $\tau < \tau_{cd}$      $\rightarrow$  deposition                      with:  $\tau_{cd}$  = critical shear stress for deposition  
 if  $\tau_{cd} < \tau < \tau_{ce}$      $\rightarrow$  simple transit                       $\tau_{ce}$  = critical shear stress for erosion  
 if  $\tau > \tau_{ce}$      $\rightarrow$  erosion

The deposition rate D (kg/m<sup>2</sup>/s) is expressed as follows:  $D=W.C.(1- \tau / \tau_{cd})$

with:        W = sediment settling velocity (m/s)

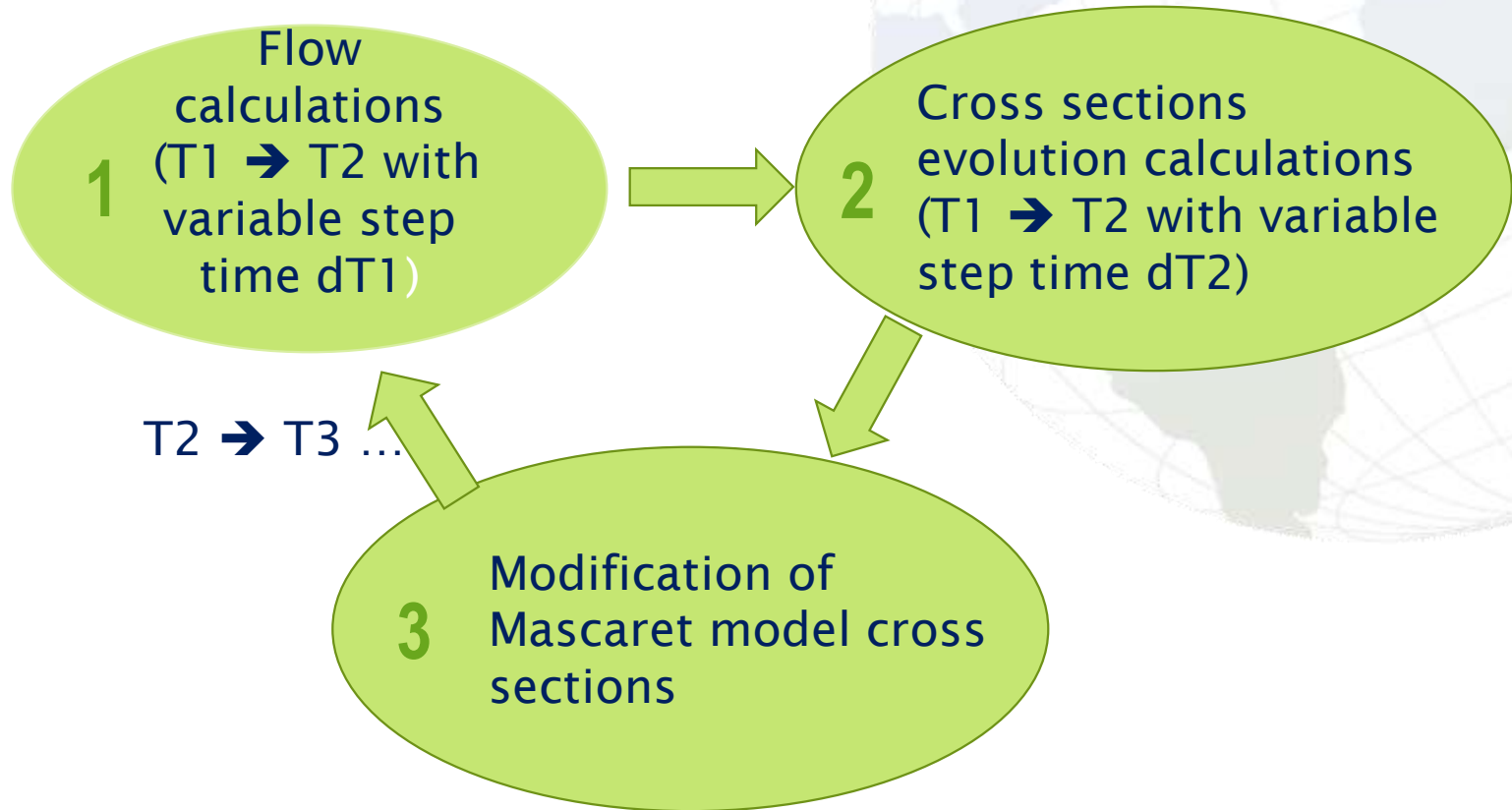
              C = concentration of suspended sediment transported in the water

The eroded sediment discharge E (kg/m<sup>2</sup>/s) is expressed by :  $E=M.(\tau - \tau_{ce})$

with:        M = coefficient (s/m)

$\tau_{ce} = \tau_{eF}$     if fresh sediment

$\tau_{ce} = \tau_{eC}$     if consolidated sediment    [ $\rightarrow$ Extreme bottom unerodable]

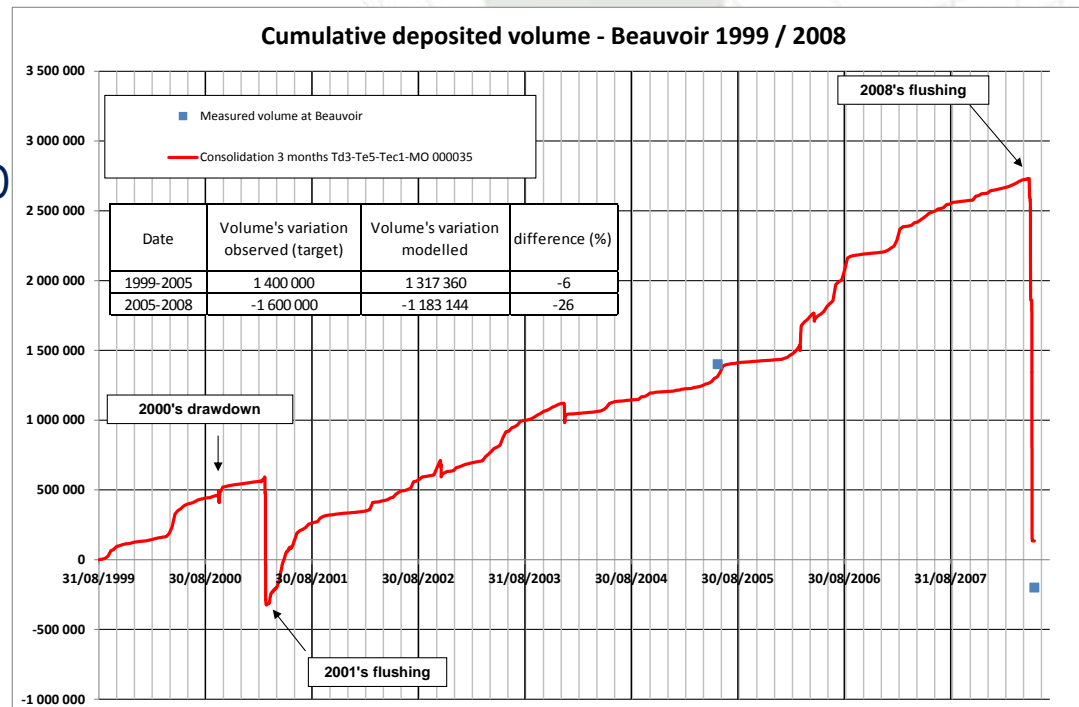


- to represent the following, as accurately as possible (from 1999 to 2008) :
  - ✓ Sediment volumes transiting through the reservoir during flushing
  - ✓ Sediment load variations over time
  - ✓ Changes reservoir bathymetry
- The results were satisfactory as the differences were between 0 and 26%
- Therefore tendency for overestimation erosion at the start of flushing
- Does not take into account 2D and 3D phenomena and bank collapse (compensation of overestimation at the start)

$\tau_{cd} = 3 \text{ Pa}$  ■  $W = 0.00035 \text{ m/s}$

$\tau_{ceF} = 5 \text{ Pa}$  ■  $\tau_{ceC} = 11 \text{ Pa}$  ■  $M = 0.0035$

$T_{consolidation} = 3 \text{ months}$



- 6 management scenarios defined and simulated over 20 years (+ 10 years for running-in)
  1. Without flushing
  2. Without dams } = boundaries of the system
  3. Present situation (= reference)
  4. More frequent flushing (Thresholds decreased from 500 to 400 m<sup>3</sup>/s)
  5. Partial drawdowns
  6. Flushing during rising flood levels (instead of decreasing f.l.)
- Analyses :
  - ➔ Sediment volumes stored in the reservoirs
  - ➔ Solid discharge
  - ➔ concentration



- 1. Without flushing
  - ➔ Huge storage and important destocking during floods
- 2. Without dams
  - ➔ Deposition in the river Rhône increases
- 3. More frequent flushing
  - ➔ Minimizes storage in reservoir
- 4. Partial drawdowns
  - ➔ Not effective
- 5. Flushing during rising flood levels
  - ➔ The best to reduce solid discharge and concentration

**The solution is to mix the 2 scenarios 4 and 5**



# THANK YOU FOR ATTENTION

- SOME QUESTIONS ?.....