

# Scour monitoring around offshore jackets and gravity based foundations

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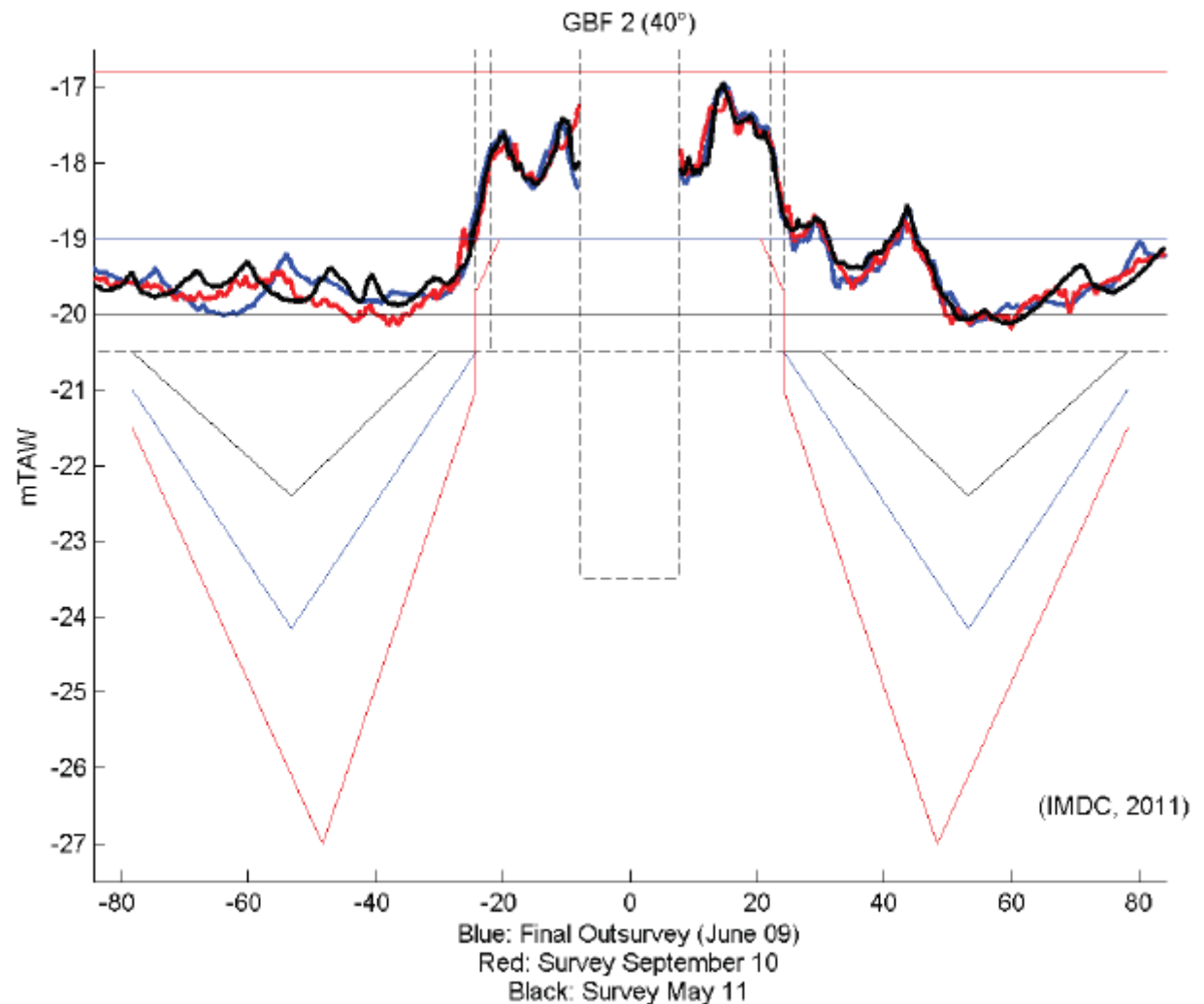
ICSE 6, August 2012, Paris



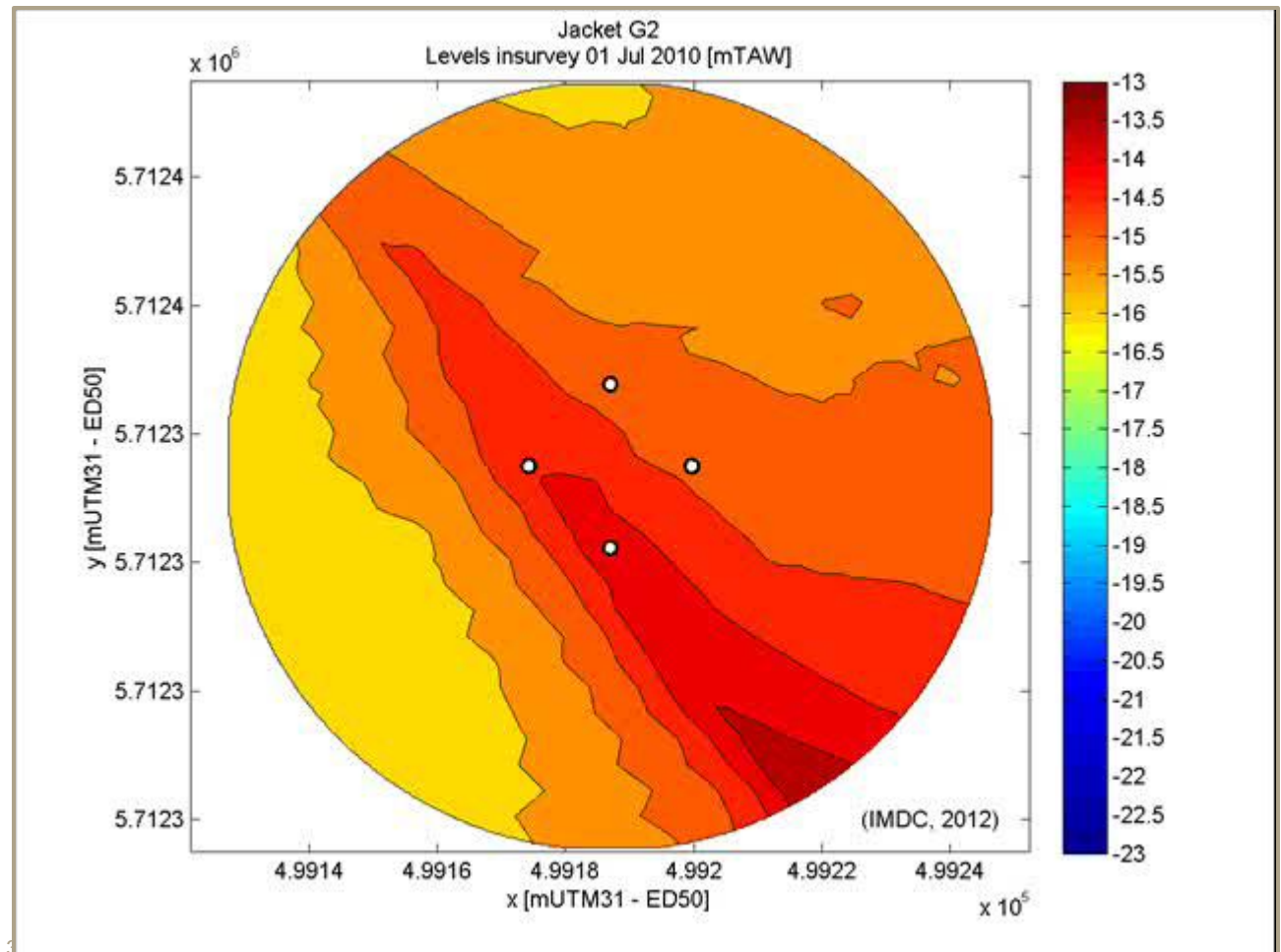
**IMDC**

International Marine & Dredging Consultants

# Sometimes, long-term measurements can become a bit boring...



... but other times you can not wait to see the result!



# Scour monitoring around offshore jackets and GBFs



- The C-Power wind farm
- Phase 1: the GBFs
  - Monitoring 2009 - 2012
- Phase 2: the jackets
  - The foundations
  - Installation
  - Predicted scour pits
  - Actual scour pits
- Conclusions

# The C-Power wind farm



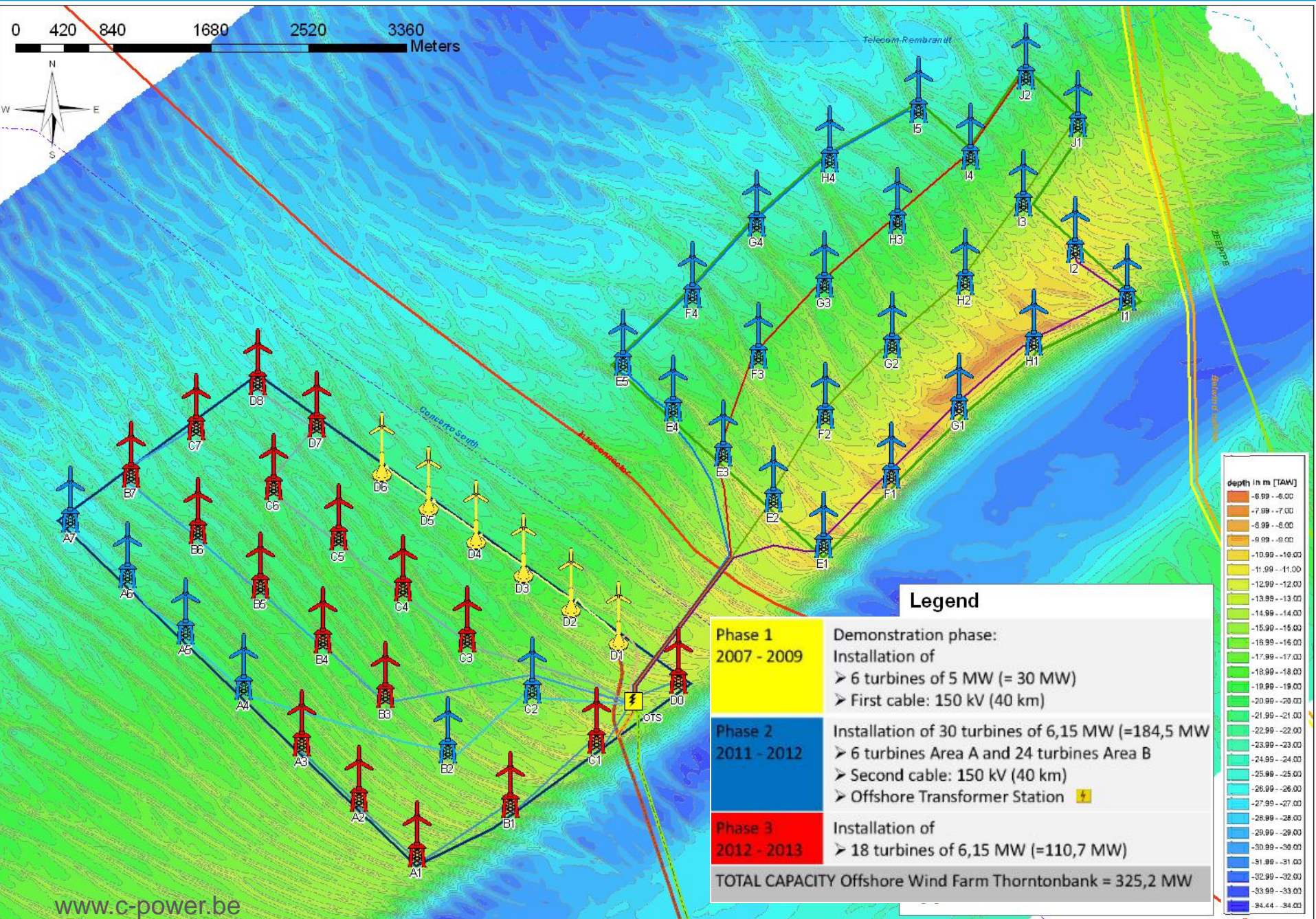


# The C-Power wind farm





# The C-Power wind farm





## Involvement of IMDC

- Owners engineer
- Design basis + scour & scour protection
- Supervision of the works
- Monitoring program

G. Dewaele

→ C-Power

J. De Winter, W. Goossens

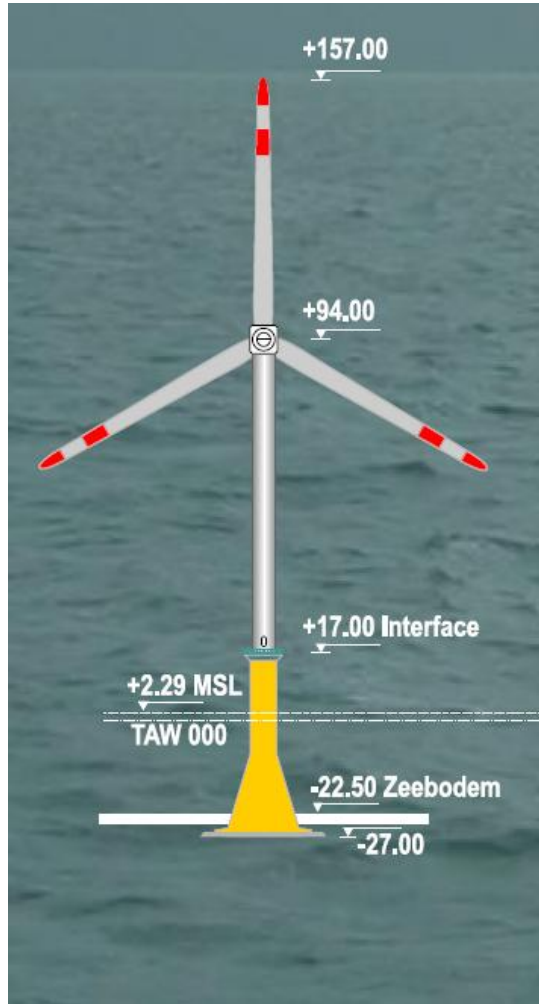
→ supervision

A. Bolle, P. Haerens

→ design issues



# Phase 1: Gravity based foundations (GBFs)



The GBF is a concrete cylindrical/conical structure, held in place by its own gravity.

Static scour protection has been placed:

- details in Bolle et al. 2009 & 2010

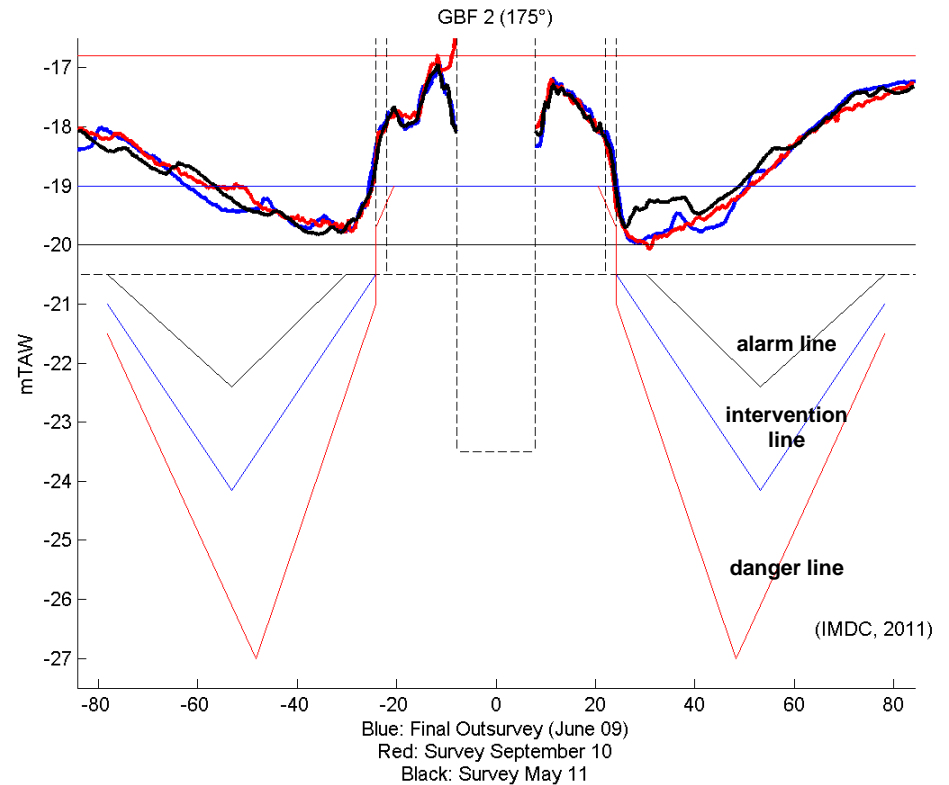
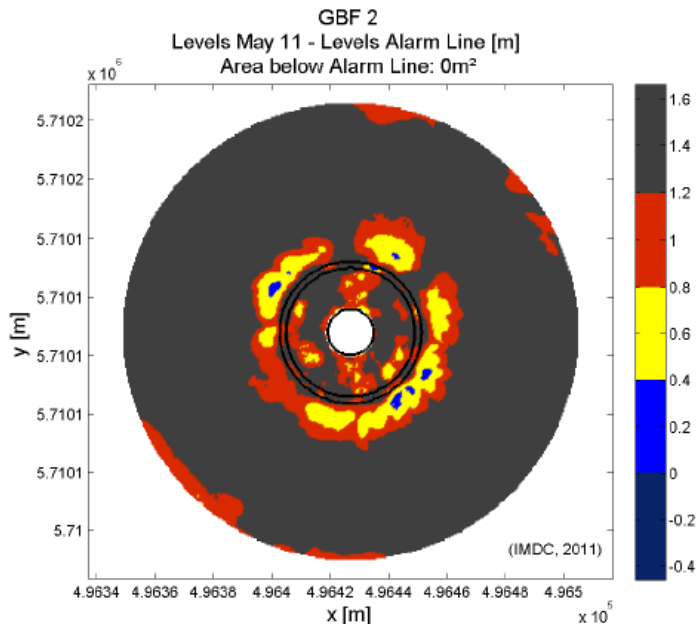
Operation and Maintenance program:

- discussed previously in IMDC (2010) and Whitehouse et al. (2011)
- multi-beam surveys at least every 6 months
- comparison with monitoring lines: alarm, intervention and danger line

# Phase 1: Gravity based foundations (GBFs)

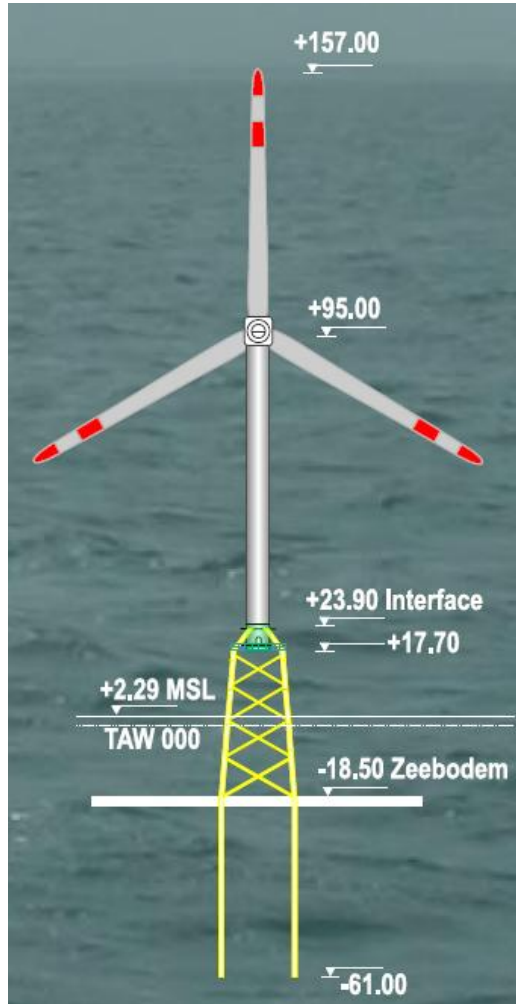
## Observations

- Armour layer is stable
- No damage observed
- No significant edge scour
- No interventions needed
- Monitoring continues



# Phase 2: Jacket foundations

## 1. The foundations



The jacket foundation is a steel structure with four legs connected to each other with braces.

The legs are grouted to pinpiles, which are driven into the sea soil.

The main advantages compared to GBFs:

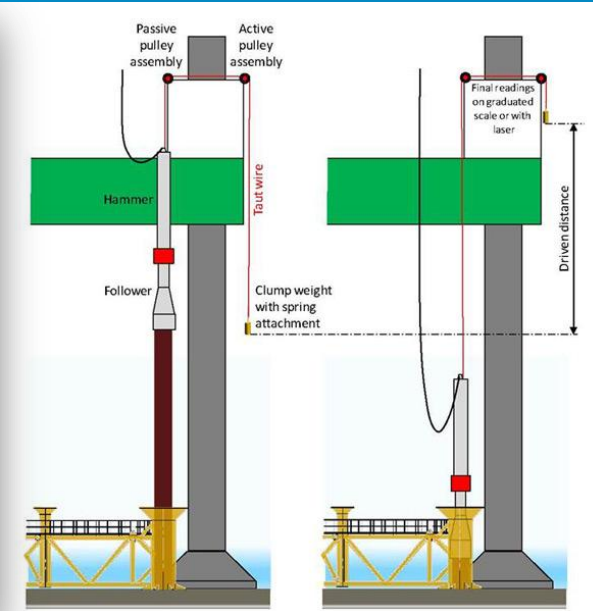
- serial production: faster fabrication & better quality control
- easier logistics: less harbour space & marine preparation works needed
- only 2 types of installation vessels are required: pre-piling & jacket installation
- more cost-effective (steel price evolution!)
- easier decommissioning



# Phase 2: Jacket foundations



## 2. The installation



# Phase 2: Jacket foundations

## 3. The predicted scour pits

### global scour depth

- $S_G = 0.37 \times D_{\text{calc}}$
- based on a 2x2 pile group (Sumer and Fredsoe, 2002)
- $D_{\text{calc}}$  = pile diameter incl. marine growth (DNV, 2007)

### global scour extent

- radius  $r_G = S_G / \tan \alpha$
- with  $\alpha$  = equal to  $\phi/2$  and  $\phi$  = the friction angle of the soil [°]

No global scour if the distance between the pile centres is more than  $6 \times D_{\text{calc}}$  (Breusers, 1972 and Hirai and Kurata, 1982)

### local scour depth $S_L$

- expected value
- maximum value

$$S_{L,e} = 1.3 \times D_{\text{calc}} \text{ (DNV, 2007 )}$$

$$S_{L,m} = 2 \times D_{\text{calc}} \text{ (Sumer et al., 2002)}$$

### local scour extent $r_L$

- expected radius
- maximum radius

$$r_{L,D} = \frac{1}{2} D_{\text{calc}} + S_{L,e} / \tan \alpha$$

$$r_{L,D} = \frac{1}{2} D_{\text{calc}} + S_{L,m} / \tan \alpha$$

with  $\alpha_{\text{downstr}} = 0.5 * \alpha_{\text{upstr}}$

- applied all around the piles (Hoffmans and Verheij, 2007)
- inclined members and secondary structures increase the turbulence



### total scour depth

- expected total scour depth
- maximum total scour depth

$$ST,e = SG + SL,e = 2.6m$$

$$ST,m = SG + SL,m = 4.1m$$

### total scour extent

- expected radius
- maximum radius

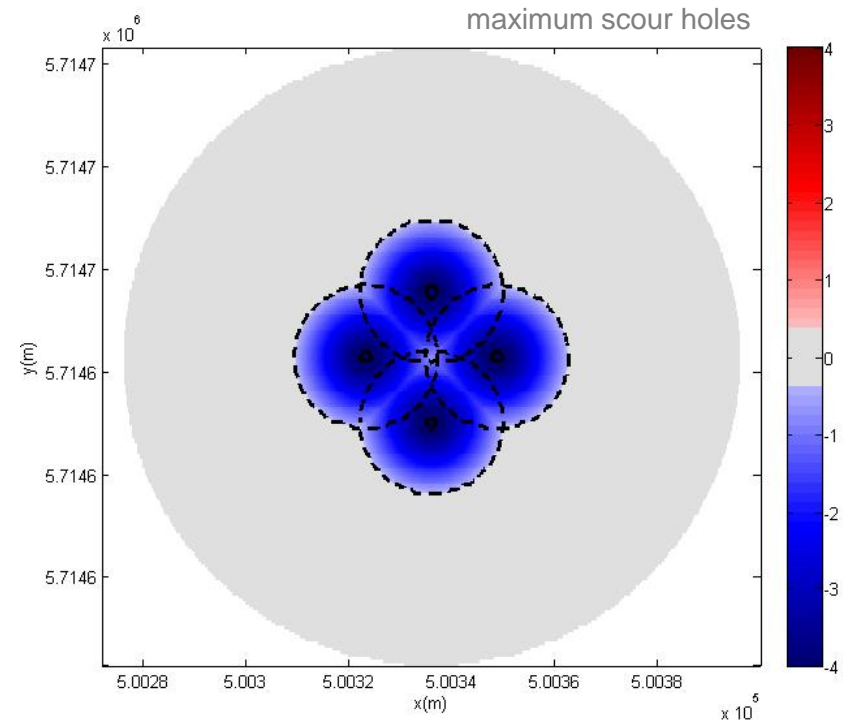
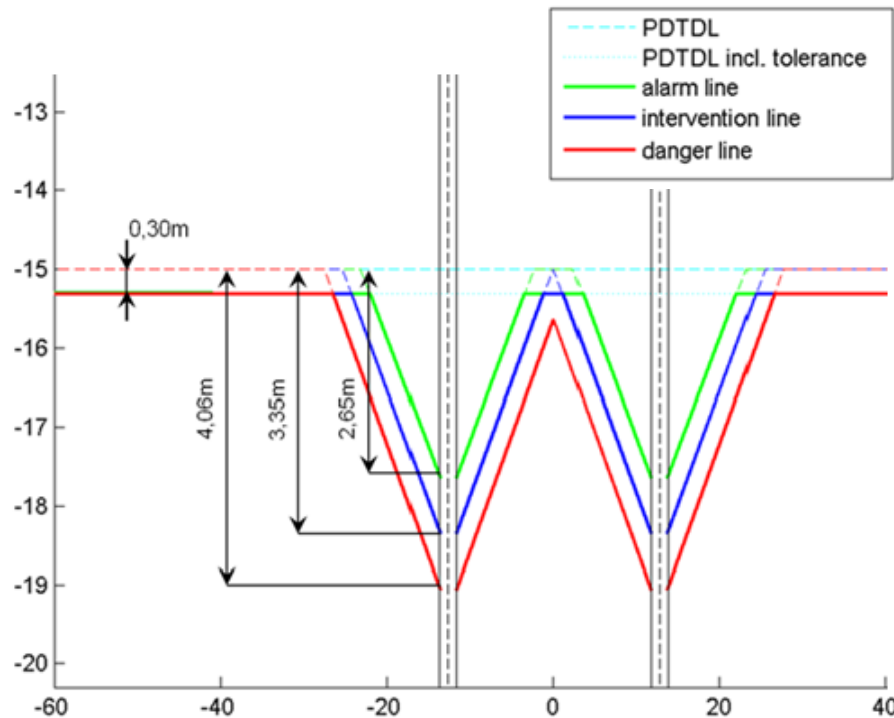
$$r_{T,e} = \frac{1}{2} D_{calc} + S_{T,e} / \tan \alpha = 9.4m$$

$$r_{T,m} = \frac{1}{2} D_{calc} + S_{T,m} / \tan \alpha = 13.9m$$

→ In this case the total scour depth and extent equals the local values, since no global scour has been found.

# Phase 2: Jacket foundations

## 3. The predicted scour pits



RSBL = Reference Seabed Level or the lowest expected level over the lifetime, without structures  
PDTDL = Pile Design Tolerance Dredging level or the lowest value of RSBL & dredged level  
alarm = expected scour depth  
danger = maximum scour depth

### Available measurements

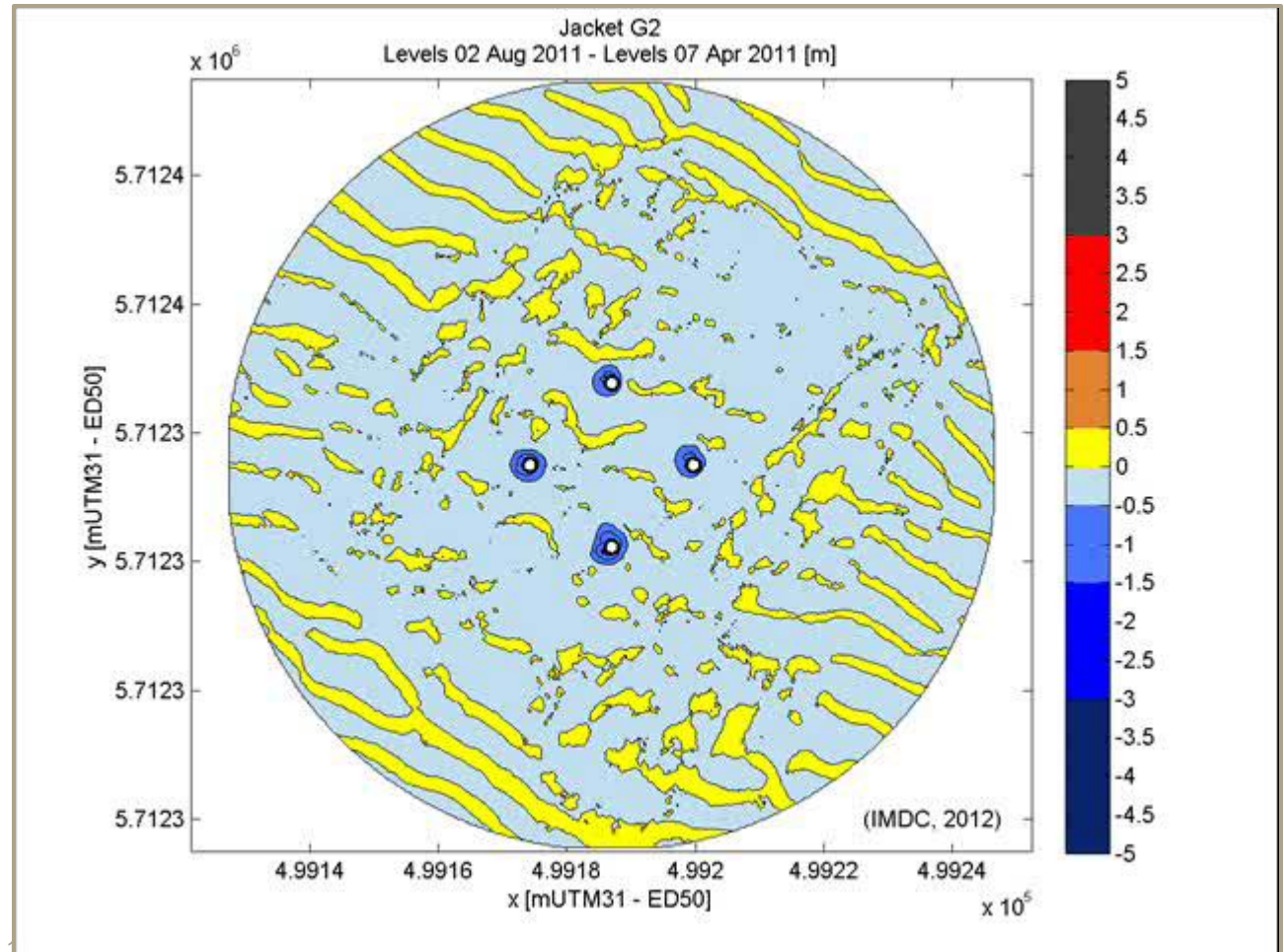
- Multi-beam surveys: from 6 up to 16 datasets per jacket
  - before dredging: August 2010 – March 2011
  - after dredging: March – April 2011
  - after pre-piling: June – September 2011
  - during cable installation: October – December 2011
  - during the first winter: December 2011 – February 2012
  - spring and summer 2012
- Hydrodynamic data
  - from the Flemish banks monitoring network



# Phase 2: Jacket foundations

## 4. The actual scour pits

### Evolution from dredging level



# Phase 2: Jacket foundations

## 4. The actual scour pits

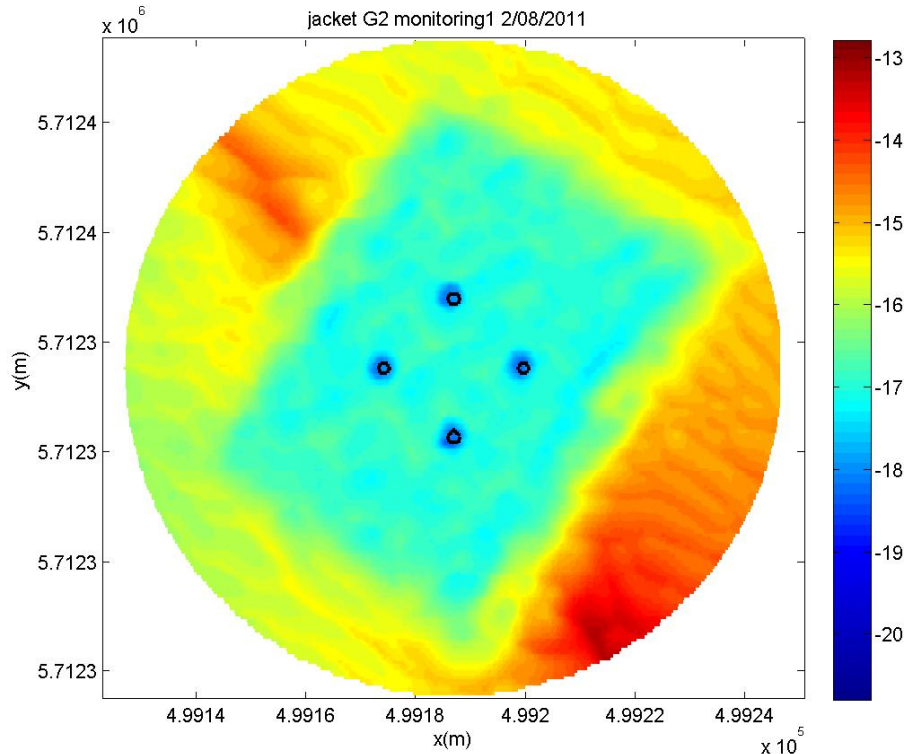
### 2.5 months after pre-piling, before jacket installation

distinct circular scour holes

- $S_{av} = 1.3\text{m}$  (0.65D)
- $S_{max} = 2.4\text{m}$  (1.2D)
- fully developed scour after 1 month (DNV, 2007; Sumer and Fredsoe, 2002)

4 piles only, pile-stick-up = 1.5m

- effect of the pile height (DHI & Snamprogetti, 1992)
- $S_{exp} = 0.9\text{m}$  (0.45D vs. 0.65D)
- $S_{max} = 1.4\text{m}$  (0.7D vs. 1.2D)
- lower than the observed values!



# Phase 2: Jacket foundations

## 4. The actual scour pits

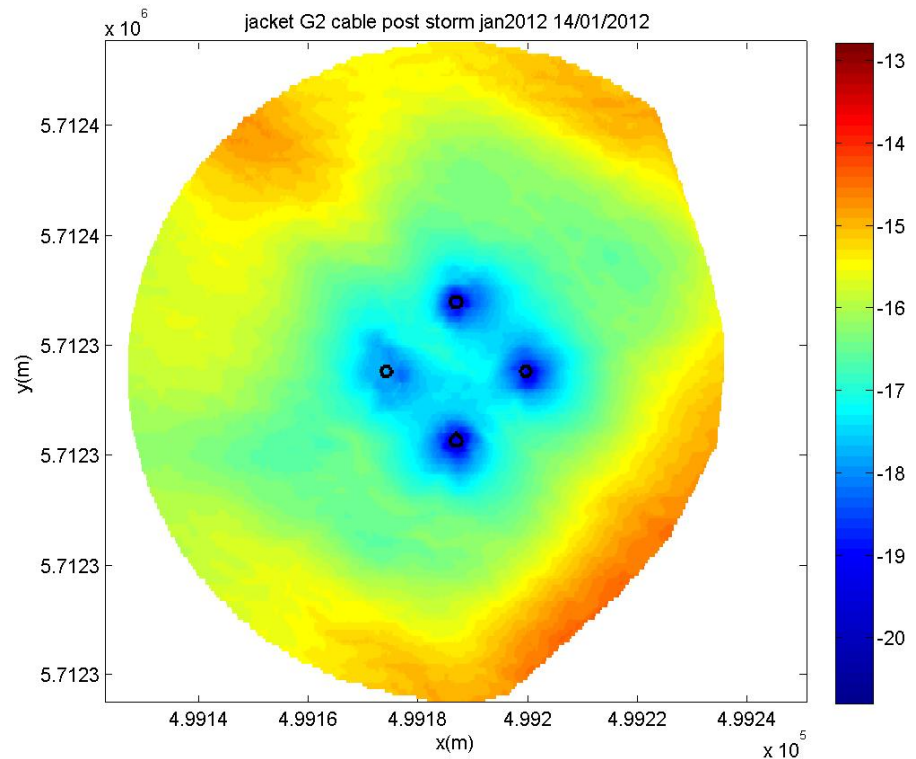
After jacket installation: October 2011 – February 2012

Observed scour:

- The depth increases instantly
  - $S_{av} = 1.4$  to  $1.9\text{m}$  ( $0.7 - 0.95D$ )
  - $S_{max, av} = 1.7$  to  $2.7\text{m}$  ( $0.85 - 1.35D$ )
- The width increases during time

Predicted scour depths:

- $S_{exp} = 2.6\text{m}$
- $S_{max} = 4.1\text{m}$

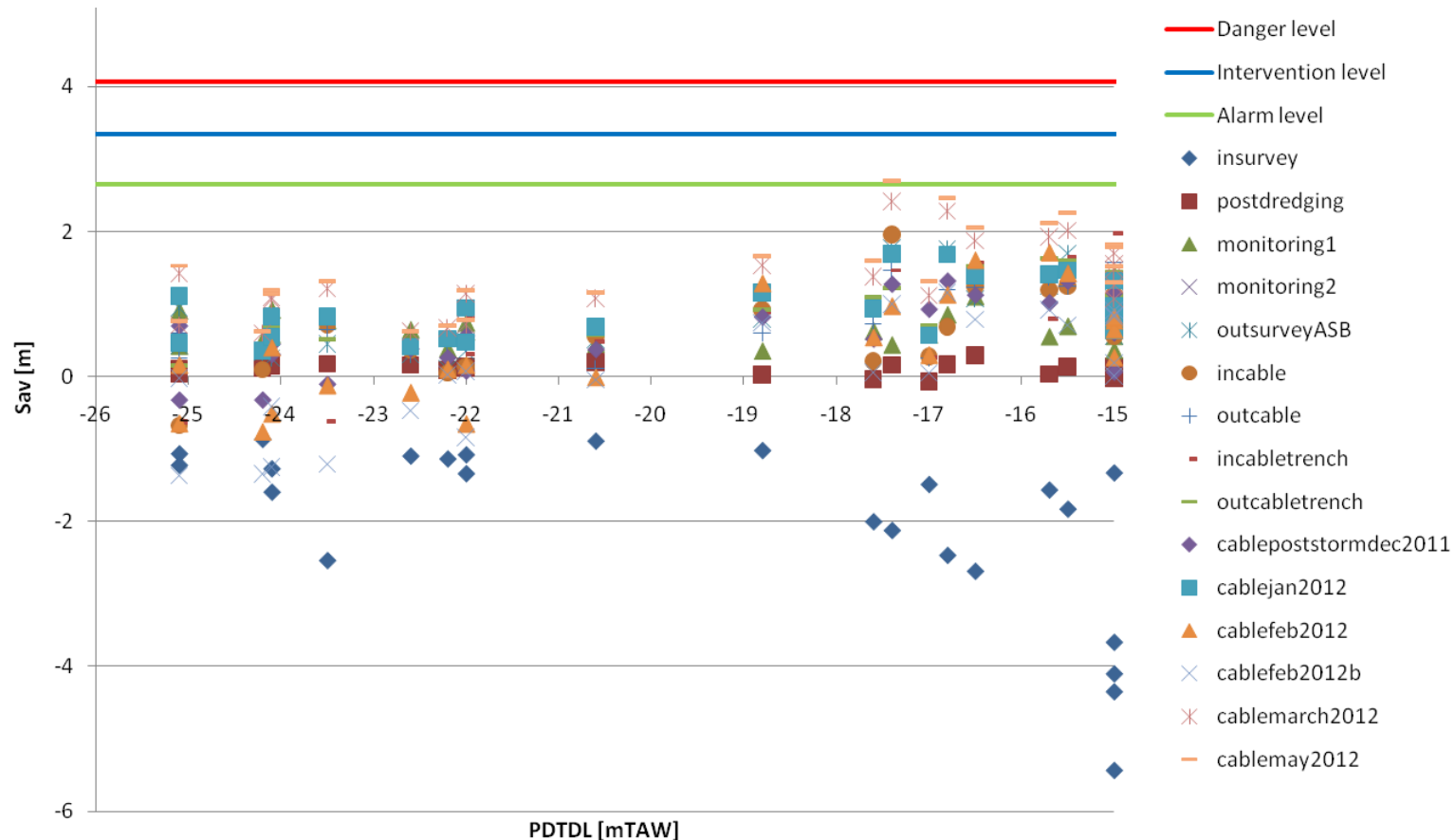




# Phase 2: Jacket foundations

## 4. The actual scour pits

### Observed scour depths



# Conclusions

**GBFs:** the monitoring went on the last years, and no damage of the scour protection was observed.

**Jacket foundations:** comparison theoretical & observed scour

- Design made for the maximum expected scour for a pile group
- The observed scour pits are close to the theoretically expected scour (or alarm level)
- Observed scour is somewhat deeper than the (average) values from literature

Monitoring continues.

# Recommendations

- Be careful when applying formulas (also DNV guideline)
- Continue monitoring & compare with data from other sites
- Combine observations with hydrodynamic conditions to obtain a new formula