
Erosion Resistant Construction of Overflow Sections by means of Geosynthetic Concrete Mattresses

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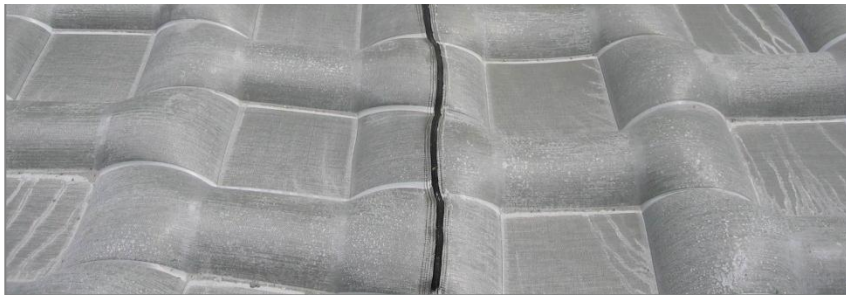
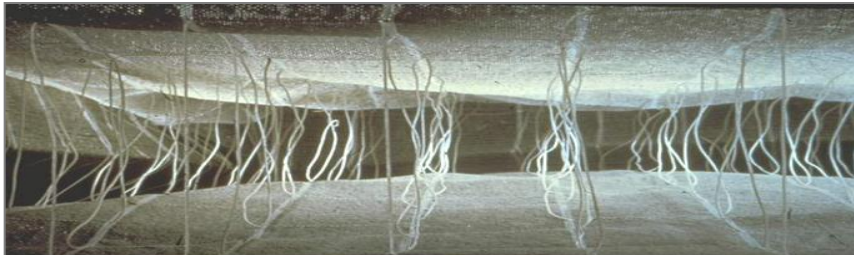
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What are concrete mattresses?

„Geotextile lost formwork“

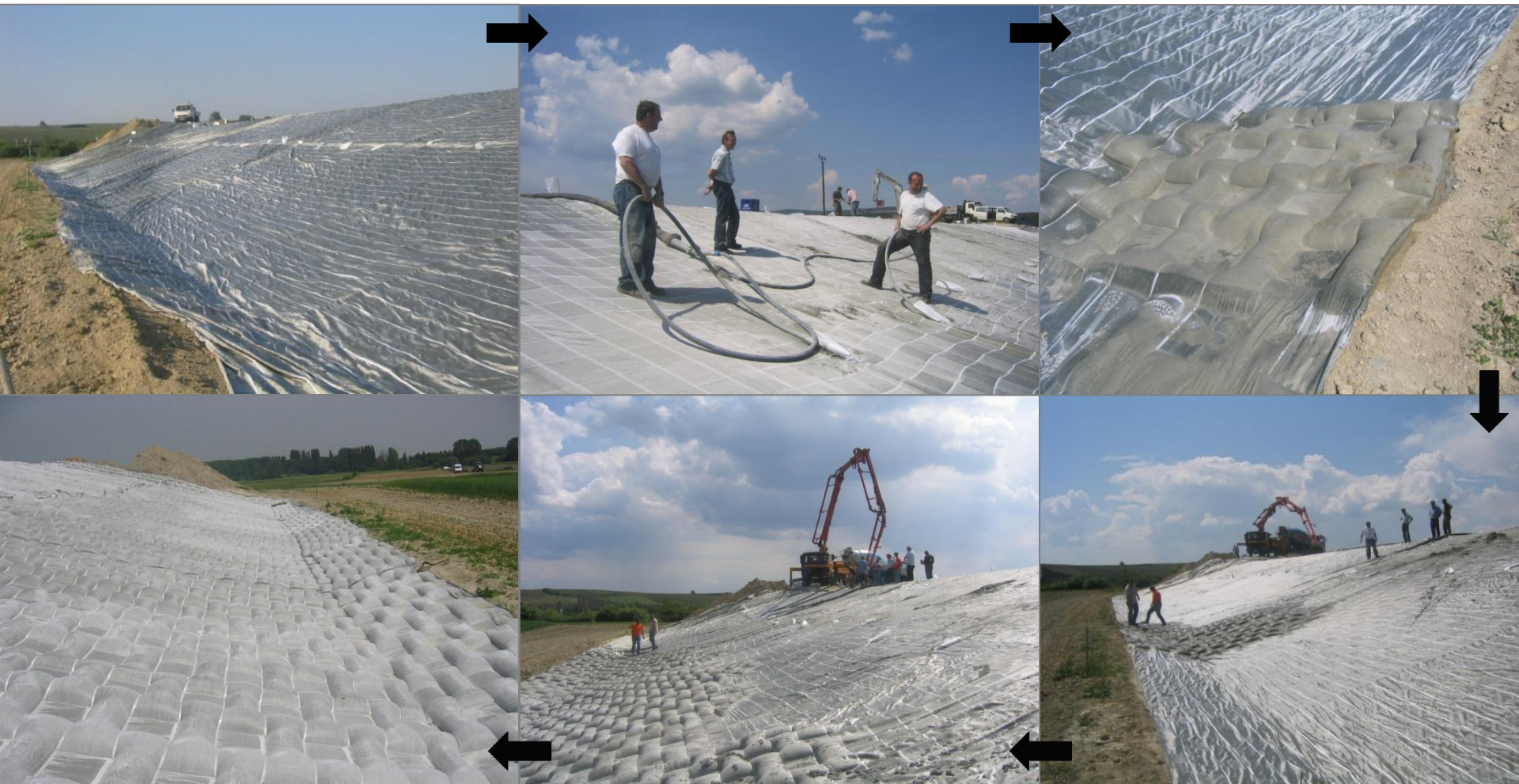


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Flowable concrete/mortar
Min. consistency class \geq F5
Preferable F6 or SCC



System explanation



Installation process

System explanation

2008



Overflow section after completion of concreting

2010



After greening and two vegetation periods

Experimental procedure

Experimental studies at the Technical University Vienna with two types of concrete mattresses

- # Recently concrete mattresses are not mentioned in literature as revetment for overflow sections → Approval of applicability
- # Verification/check of the maximum permissible flow velocities for concrete mattresses given in recent literature



Filterpoint (FP) mat



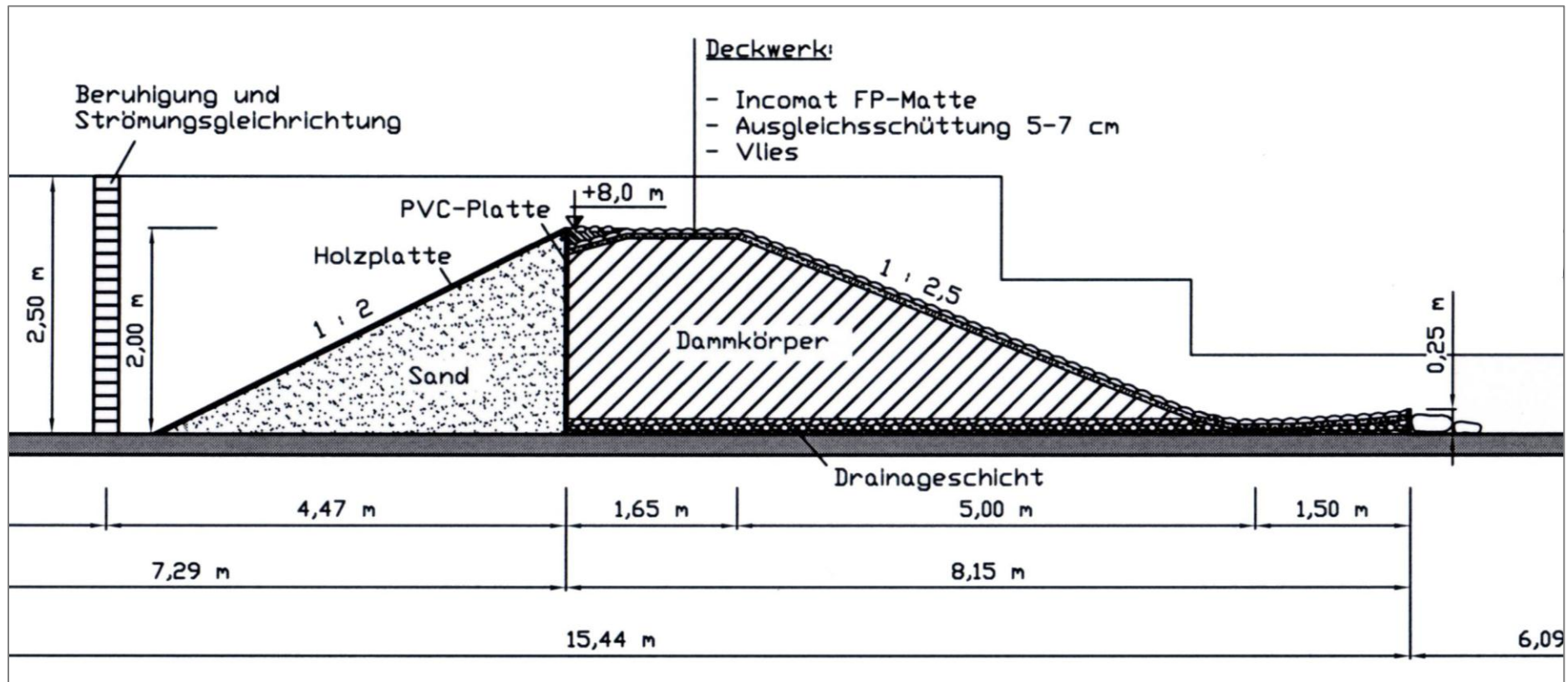
Crib mat



Experimental procedure

Experimental set-up

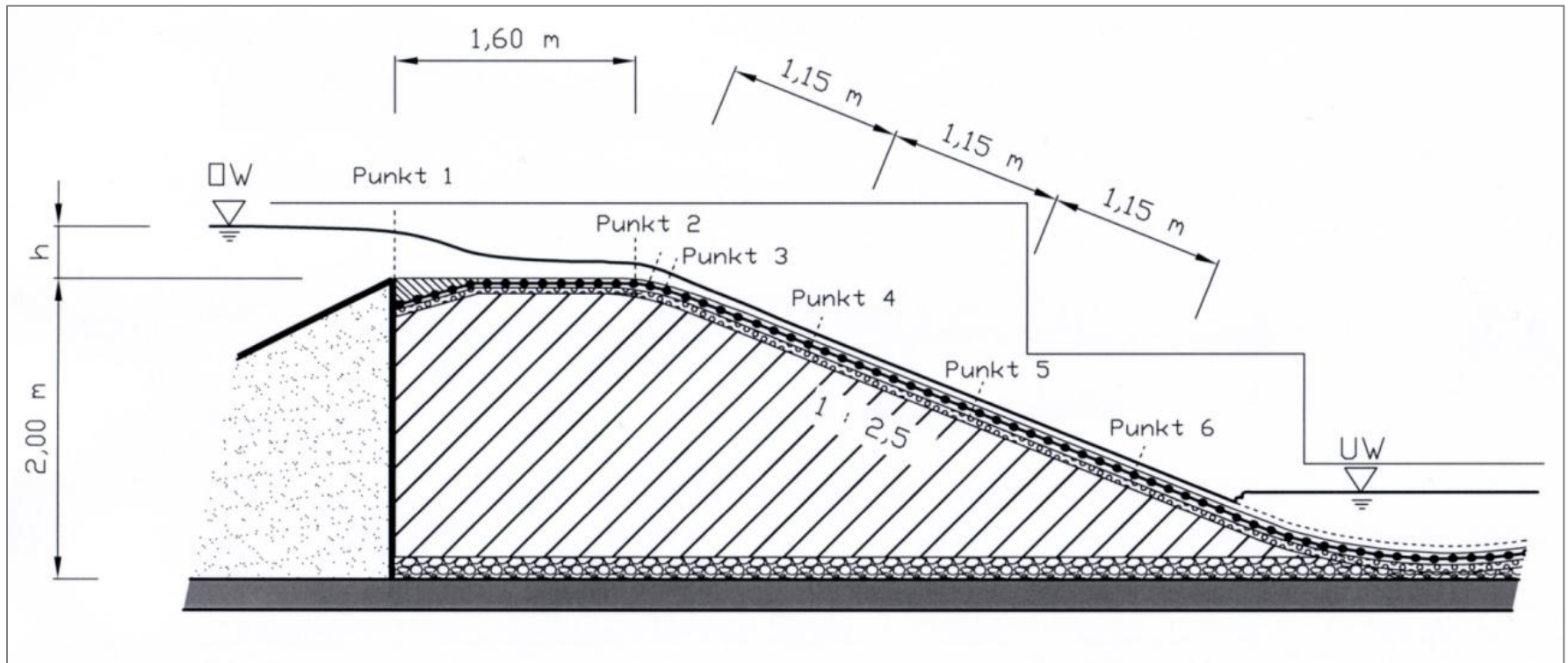
- # Physical model with the scale of 1:4
- # Froude's model law
- # Slope inclination 1:2,5
- # Specific discharge $0,5 \dots 2,5 \text{ m}^3/(\text{s} \cdot \text{m})$



Experimental procedure

Location of the measuring points

- # Flow velocity
- # Water depths



Experimental procedure



q [m³/(s*m)]	Duration [h]	
	Filterpoint- mat	Crib- mat
0.50	10	14
1.00	14	15
1.50	11	15
2.00	17	72
2.25	0.5	0.5
2.50	0.5	0.5

Overflow tests at the TU Vienna 2010

$q = 0,25 \text{ m}^3/(\text{s} \cdot \text{m})$



$q = 0,50 \text{ m}^3/(\text{s} \cdot \text{m})$

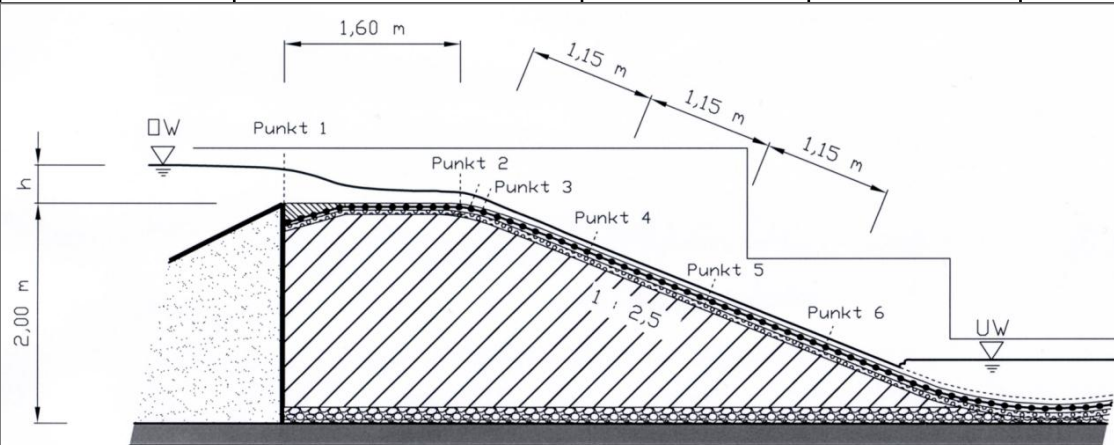


$q = 2,50 \text{ m}^3/(\text{s} \cdot \text{m})$



Experimental procedure

	Specific discharge q	Measured maximal flow velocity					
Type of mat	$[m^3/(s \cdot m)]$	$[m/s]$					
		Point 1	Point 2	Point 3	Point 4	Point 5	Point 6
FP	0.50	1.30	2.80	3.20	5.80	5.66	5.60
	1.00	1.70	3.64	4.32	6.80	7.80	7.80
	2.00	2.20	4.20	4.88	7.26	9.16	9.50
Crib	0.50	1.45	3.16	4.30	6.44	6.00	6.68
	1.00	1.84	3.62	4.52	6.44	7.58	8.14
	2.00	2.30	4.16	5.04	7.36	8.96	10.50



Summary of the test results

- **Structural design of the overflow section is essential**
 - **connection to the dam crest**
 - **stilling basin/toe design**
 - **lateral integration into the dam/structure**
 - **sufficient drainage layer below the concrete mattress**
 - **stable subsoil (well compacted)**

- **No indication for a failure – neither for the Crib nor for the FP mat**
 - **for great discharges ($q=2,0\text{m}^3/(\text{s}\cdot\text{m})$)**
 - **for a steep slope (1:2,5)**
 - **for hydraulic loads over a long period of time (72 h)**
 - **for higher hydraulic loads over a short period of time ($q=2,25\ldots2,5\text{m}^3/(\text{s}\cdot\text{m})$)**

Summary

Revetment type	Max. slope [1:n]	q_{\max} [m³/(s*m)]
Pitched stone ¹⁾	6	≤ 1.0
Rip-rap ¹⁾	4	≤ 1.0
Geosynthetic gabions ¹⁾	4	≤ 1.0
Mastix asphalt ¹⁾	6	≤ 1.0
Grass paver ¹⁾	6	≤ 1.0
Soil solidification ¹⁾	4	≤ 1.0
Filterpoint or Crib mat ²⁾	2.5	> 2.0

¹⁾ According to [LfU BW - Überströmbare Dämme und Deichscharten]

²⁾ Derived from the model tests at the TU Wien

Comparison of conventional revetment systems concerning discharge capacity and maximum permissible slope

Advantages of the concrete mattress system

- # **Very high resistance to increased hydraulic loads/discharges
(→ greater in comparison to conventional systems)**
- # **Optimization of the dam cubature
(→ steeper slopes are permissible)**
- # **Reduced layer thickness**
- # **Very economical system**
- # **With adequate preparation great daily installation rates
(→ up to 1.500 m²/d)**
- # **Subsequent protection of overflow sections is possible
(→ no change of the main dam body required)**
- # **Coherent revetment**
- # **Very pleasant appearance/integration into the landscape due to subsequent grass cover**



Thanks for your kind attention!

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